



Application Sheet

Glasgow Central Station Truss

Context: Roof truss in the main concourse of the Glasgow Central Station

Objective: Estimating the size of members in an existing truss based on bending

Concepts used in this application sheet

- Force: applied load, point load, internal force, resolution into components
- Equilibrium: equilibrium equation, force equilibrium
- Free Body Diagram

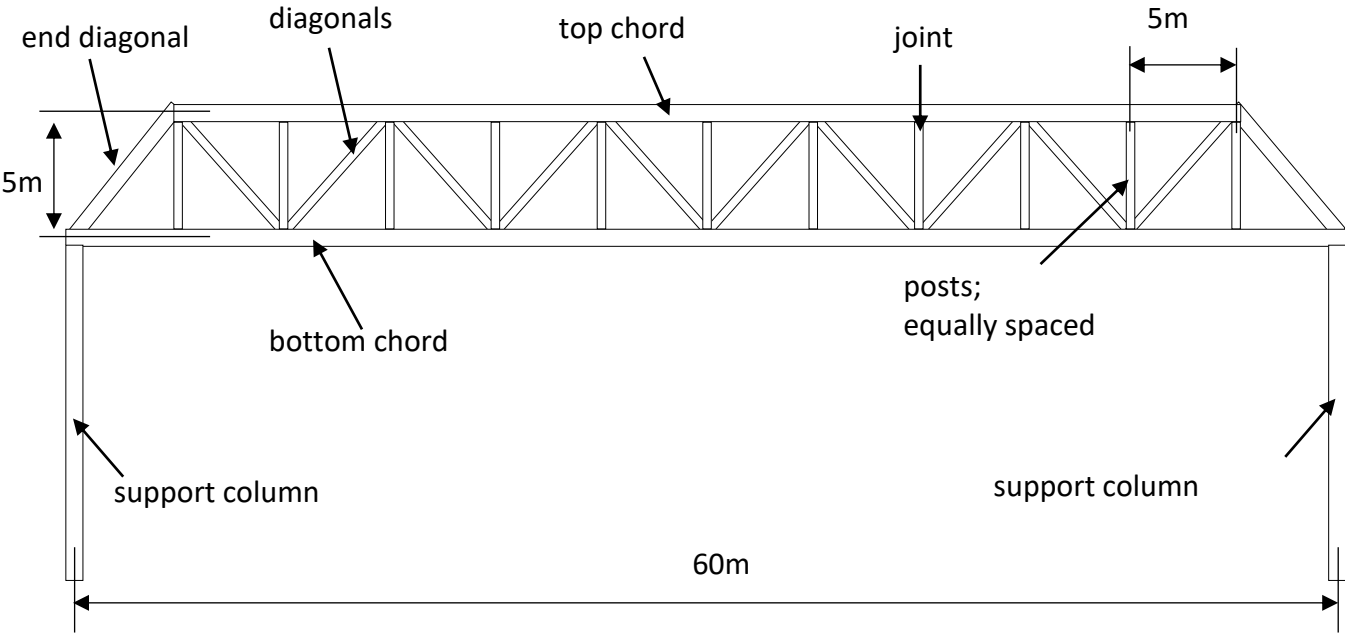


Main concourse truss (“[Glasgow Central railway station](#)” by Ed Webster, licenced under [CC BY 2.0](#))



Truss details (“[2014_276](#)” by Chilanga Cement, licenced under [CC BY 2.0](#))

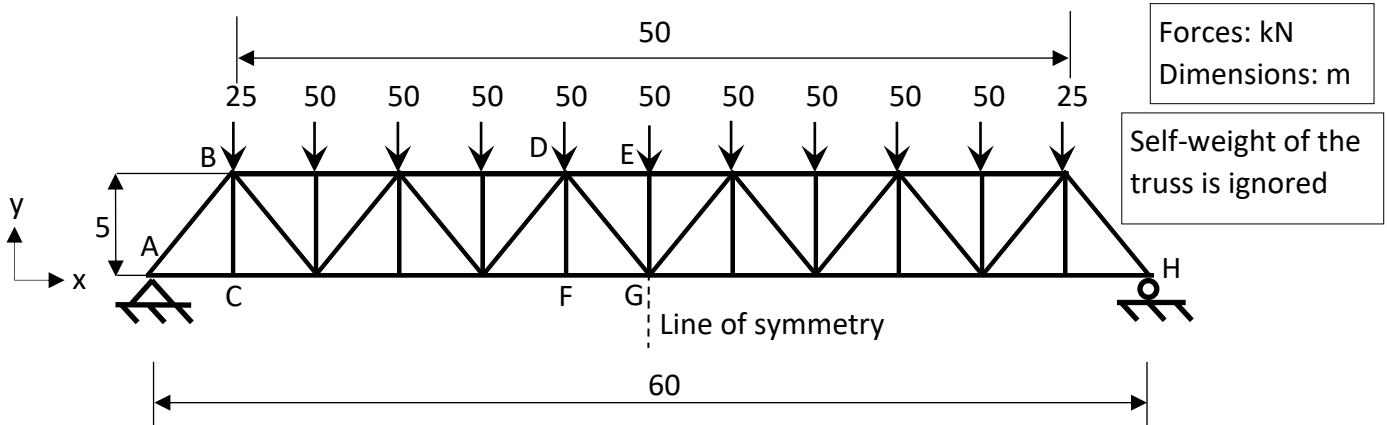
Engineering model



All sections: steel square hollow sections

Structural analysis

Analysis model



For this calculation only axial forces in the members are taken into account i.e. moment continuity is neglected.

Calculations

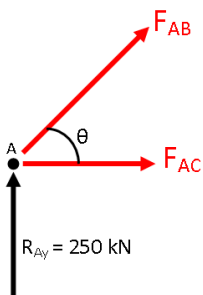
Two of the heavily loaded members are the end diagonal AB and the top chord DE at the centre of the beam.

Calculate the force in the end diagonal AB:

Calculate the support reaction at A and H:

Total load on the truss: 500 kN

Vertical support reaction = $500/2 = 250$ kN (because truss and loading are symmetric)



Use the free-body diagram at the end support:

$$\theta = \tan^{-1}(BC/AC) = \tan^{-1}(5/5) = \tan^{-1}(1) = 45^\circ$$

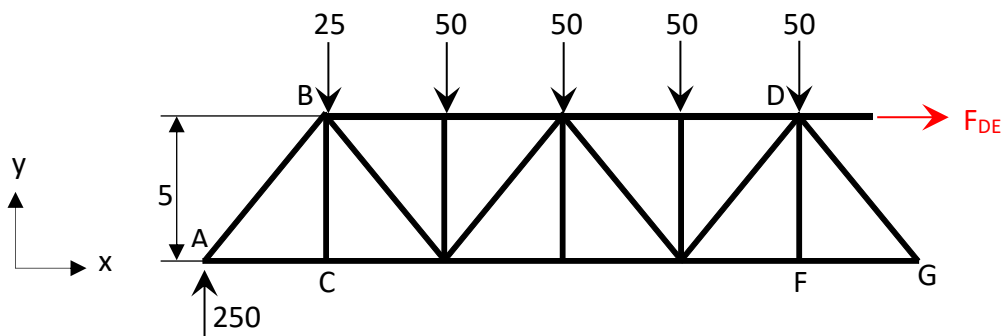
$$\sum F_y = 250 + F_{AB} \sin(\theta) = 0$$

$$F_{AB} = -250/\sin(\theta) = -250/\sin(45) = -356\text{kN}$$

F_{AB} is negative. i.e. force in AB is compressive

Calculate the force in the chords at the centre of the truss

Use a cut just to the left of the central post:



Take moment equilibrium about joint G:

$$-F_{DE} \cdot 5 - 250 \cdot 30 + 25 \cdot 25 + 50 \cdot (20 + 15 + 10 + 5) = 0$$

$F_{DE} = -875\text{kN}$ (i.e. compressive)

Assessment

Using the [allowable stress method](#)

Strength criterion $\sigma_c/\sigma_a \leq 1.0$

- σ_c is the compressive stress due to loading
- σ_a is the design allowable compressive stress

Data input

Yield stress in steel $\sigma_y = 240 \text{ N/mm}^2$ (from code of practice; 240 N/mm^2 is an older steel classification that is no longer used. However, it is used here because the truss was built in the late 1800s.)

Factor of safety on stress $FoS = 2.0$ (assumed)

Selecting an arbitrary square hollow section (from [steel tables](#)) with dimensions $200 \times 200 \times 5.0$

Area of section, $A_s = 38.7 \text{ cm}^2 = 3870 \text{ mm}^2$ (from [steel tables](#))

End diagonal AB assessment

AB specific data input

Axial force $F_{AB} = 356 \text{ kN} = 356000 \text{ N}$ (from analysis)

Calculations

Allowable stress in steel $\sigma_a = \sigma_y/FoS = 240/2.0 = 120 \text{ N/mm}^2$

Compressive stress in member $\sigma_c = F/A = 356000/3870 = 92.0 \text{ N/mm}^2$

Apply the criterion

$$\sigma_c/\sigma_a \leq 1.0 = 92/120 = 0.77$$

Decision

The chosen steel section is suitable for the compressive forces it has to withstand. Also, the selection is not oversized as the unity factor is rather close to 1.0.

Top chord DE assessment

DE specific data input

Axial force $F_{DE} = 875 \text{ kN} = 875000 \text{ N}$ (from analysis)

Calculations

Allowable stress in steel $\sigma_a = \sigma_y/FoS = 240/2.0 = 120 \text{ N/mm}^2$

Compressive stress in member $\sigma_c = F/A = 875000/3870 = 226.1 \text{ N/mm}^2$

Apply the criterion

$$\sigma_c/\sigma_a \leq 1.0 = 226.1/120 = 1.88$$

Decision

The chosen section size is not suitable for the top chord. It needs to be bigger.

The next steps can either be:

- Choosing an arbitrary section and performing the same calculations as above or
- In this case it is possible to rearrange the equations to solve for the minimum required section area

Adjustment calculations

Substitute the equations for compressive and allowable stress into the unity factor equation:

$$\sigma_c/\sigma_a = (F/A_s)/(\sigma_y/FoS) = (875000/A_s) / (120) = 1.0$$

Solve for A_s

$$875000/A_s = 120 \cdot 1$$

$$A_s = 7292 \text{ mm}^2 = 72.9 \text{ cm}^2$$

Choose a section from the [steel tables](#) with an area not less than A_s :

$200 \times 200 \times 12.5$ with $A = 92.1 \text{ cm}^2$

Validation

The calculation does not allow for buckling of the members. This needs to be checked. Neglect of continuity in the analysis model will be conservative.

A spreadsheet in which the above calculations can be checked can be found [here](#)

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