



## Application Sheet

### Crane tension cable

**Context:** Design of a tower crane

**Objective:** To assess the adequacy of the cable stay for the jib.

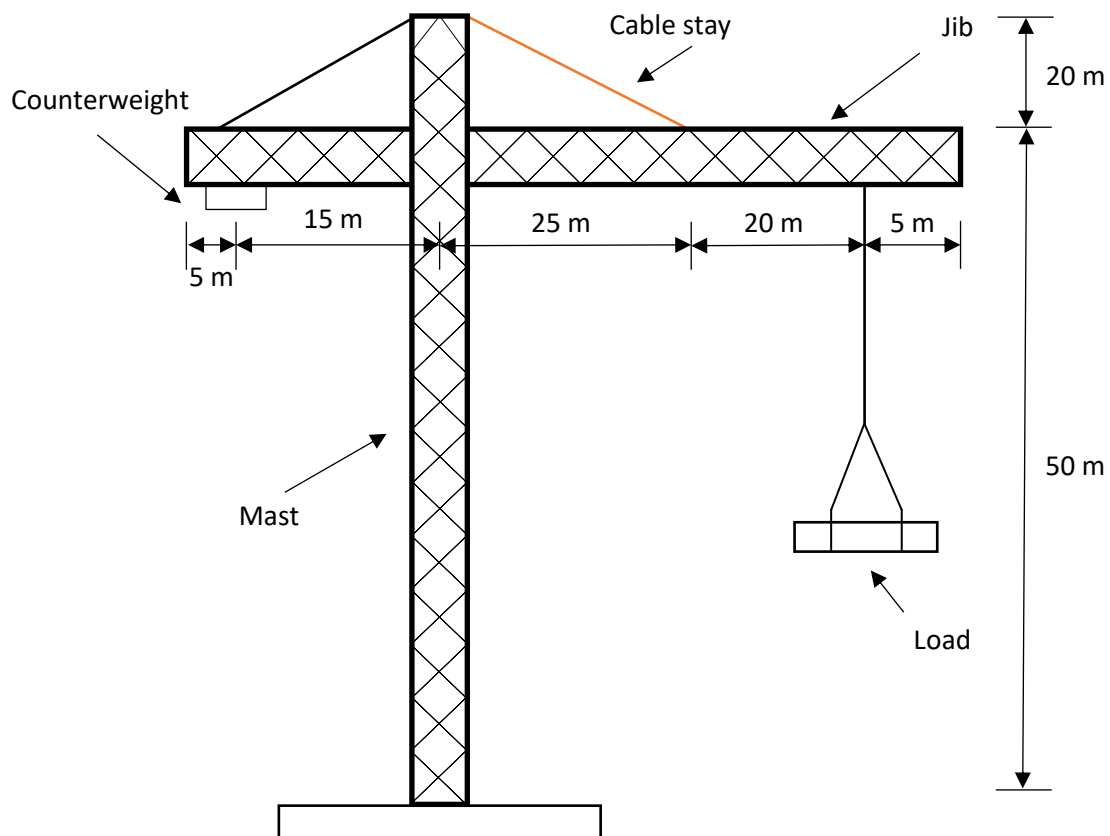
#### Concepts used in this application sheet

- Force: applied load, moments, force resolution



Figure 1 “Crane”  
(booledozer, licenced under [CC0 1.0](https://creativecommons.org/licenses/by/4.0/))

#### Engineering model



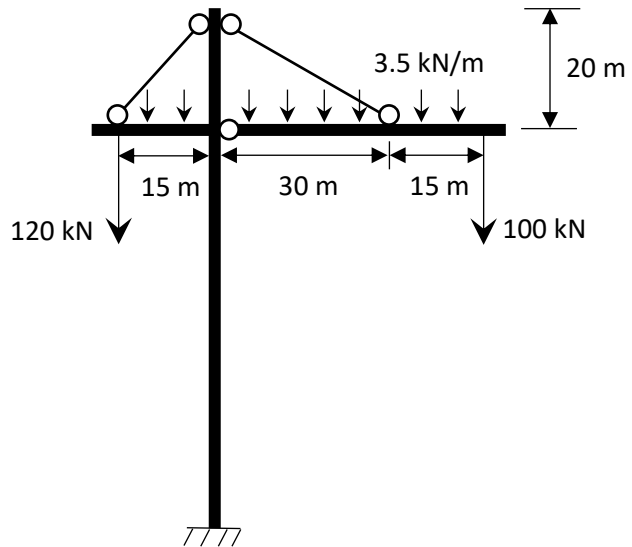
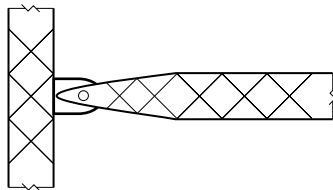
## Structural analysis

### Analysis model

The self-weight of the jib is modelled as a uniformly distributed load along the length of the jib.

As a simplifying assumption for this calculation, it is assumed that there is no moment continuity in the jib at the mast i.e. the end of the jib is pinned to the mast.

Pin connection:



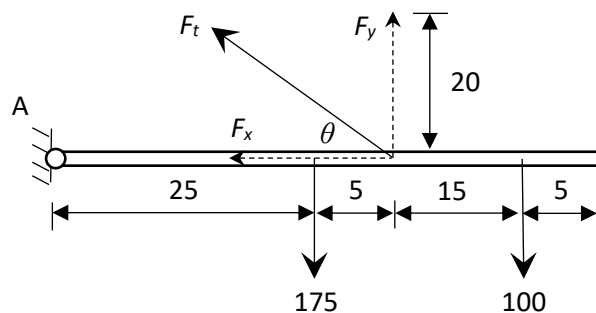
All forces and dimensions are given in kN and m respectively, unless otherwise stated.

Calculate tension force in the stay

Free body diagram of the jib:

Calculate the resultant load due to the self-weight of the jib:  
 $3.5 \times 50 = 175 \text{ kN}$  at 25 m

Take moments about A:  
 $\sum M = (100 \times 45) + (175 \times 25) - (F_y \times 30) = 0$   
 $F_y = 296 \text{ kN}$

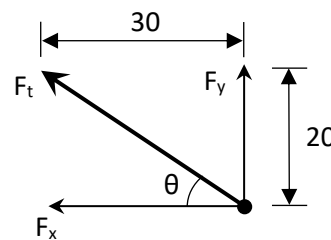


Resolve forces to find the total cable force:

Using basic trigonometry:

$$\theta = \tan^{-1}(20/30) = 34^\circ$$

$$F_t = F_y / \sin(\theta) = 296 / \sin(34) = 529 \text{ kN}$$



Apply partial factor for load

Partial safety factor for load  $\gamma_f = 1.5$  (for preliminary checking)

$$\text{Design load } N_{Ed} = F_t \times \gamma_f = 529 \times 1.5 = 794 \text{ kN}$$

## Assessment

*Strength criterion for axial force*

Use the [partial factor method](#)

$$N_{Ed}/N_{Rd} \leq 1.0$$

- $N_{Ed}$  is the design axial force in the stay due to the applied loading
- $N_{Rd}$  is the design axial resistance

$$N_{Rd} = (A f_y) / \gamma_m$$

where  $A$  is the cross-sectional area of the section

$\gamma_m$  is the partial safety factor for resistance

*Data input*

Design force  $N_{Ed} = 794 \text{ kN} = 794 \cdot 10^3 \text{ N}$

Diameter of the stay  $D = 25 \text{ mm}$

Yield stress of the stay  $f_y = 355 \text{ N/mm}^2$  (for Grade S355 steel from EN 1993-1-1 - Table 3.1)

Partial safety factor for material resistance  $\gamma_m = 1.0$  (from Eurocode)

*Calculations*

Area of stay  $A = \pi D^2/4 = \pi \cdot 25^2/4 = 491 \text{ mm}^2$

Design axial resistance  $N_{Rd} = (A f_y)/\gamma_m = (491 \cdot 355)/1.0 = 174 \text{ kN}$

*Apply the criterion*

$N_{Ed}/N_{Rd} = 794/174 = 4.6 > 1.0 \therefore$  not acceptable

Decision: cable size must be increased

*Validation*

The force in the cable may be significantly less than the estimated value because of the assumption of the pin connection for the jib. Overestimating the force is a safe assumption.

Crane collapses are relatively common and can result in deaths. Therefore, a higher than normal factor of safety is used to ensure the crane will not be overloaded.

*Note: A version of this example using the factor of safety method is available [here](#).*

## Metadata

Keywords: Force resolution, partial safety factors, moment equilibrium

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